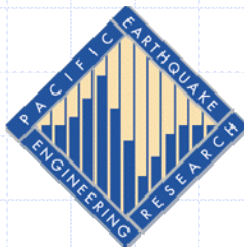


Probabilistic Seismic Design and Evaluation of Nuclear Facility Structures

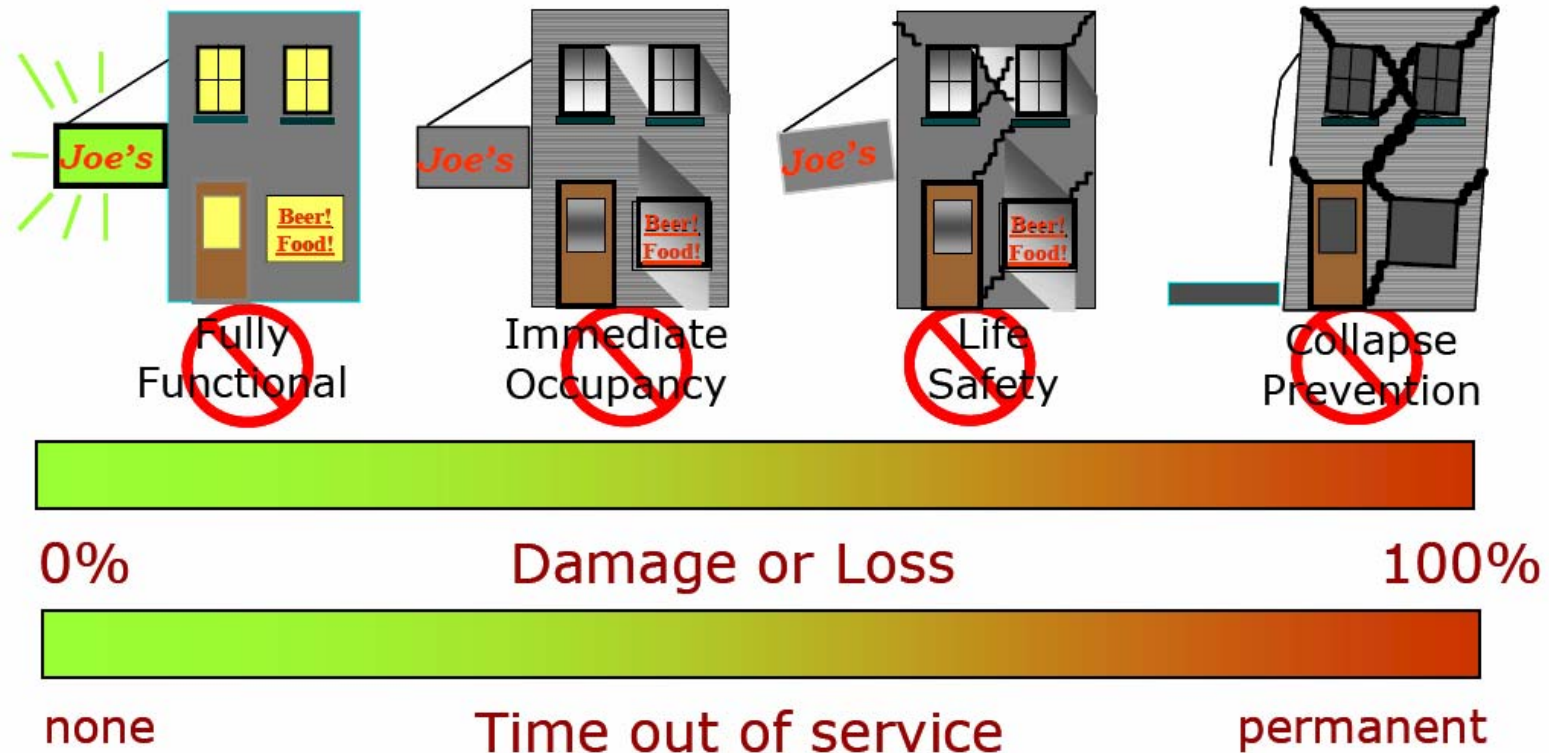
Bozidar Stojadinovic, Associate Professor



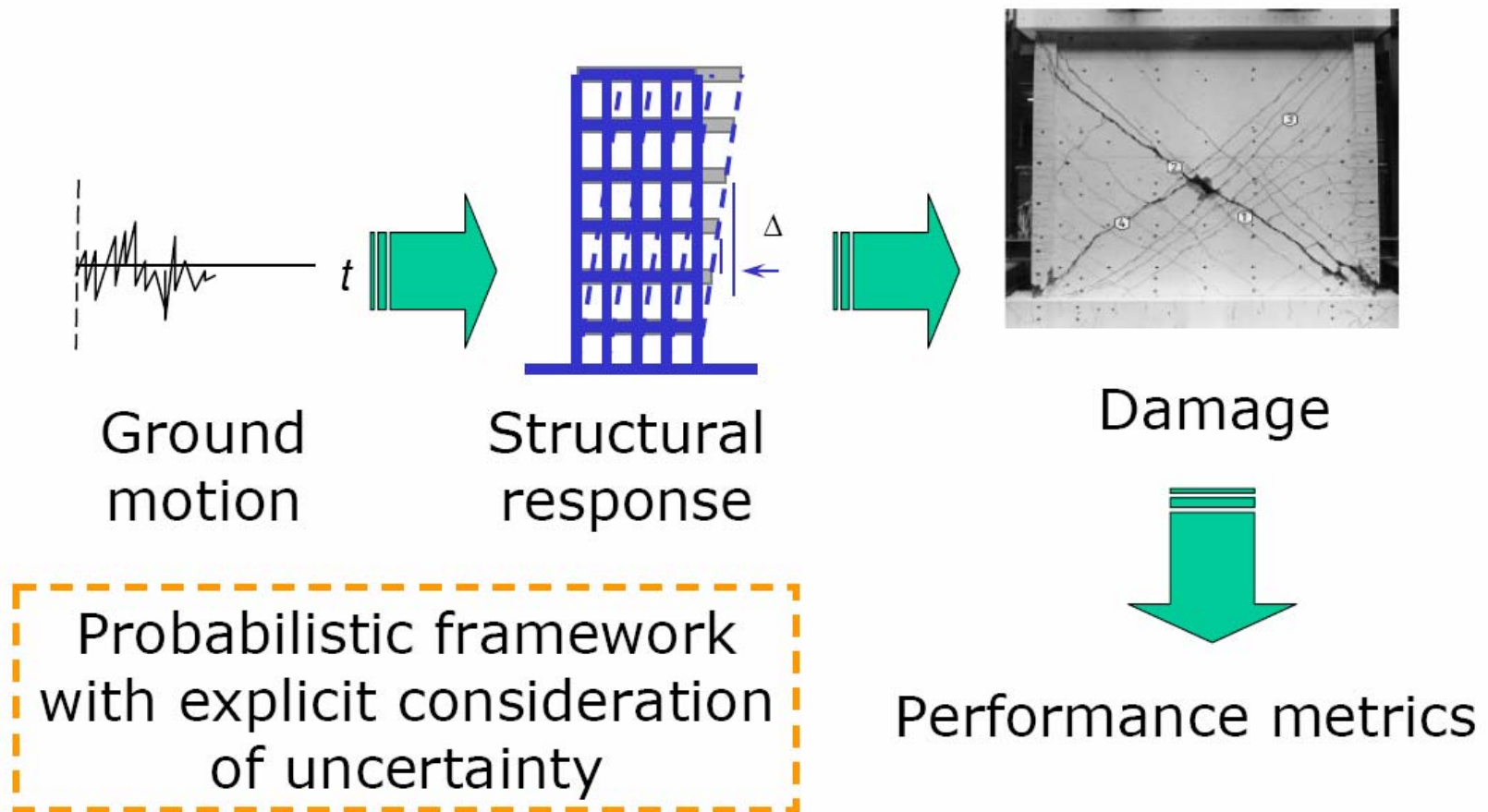
Department of Civil and Environmental Engineering
University of California, Berkeley

Pacific Earthquake Engineering Research Center

Performance-Based Design



Probabilistic Approach



Goals and Benefits

- ◆ Improve prescriptive code requirements
 - Beneficial all building types and all stakeholders
- ◆ Characterize the potential consequences of building response to earthquakes by estimating:
 - Direct economic loss: repair and replacement costs
 - Indirect economic loss: business interruption
 - Casualties: deaths and injuries
- ◆ Enable design for:
 - Better performance of critical facilities
 - Greater confidence in better performance through using new structural protection concepts, such as base isolation
 - Equivalent performance (wrt. code) but at lower cost and with higher confidence

Common Probabilistic Basis for Evaluation of Structures

- Given a seismic hazard environment and a structure, the probability that a performance objective is achieved is:

$$P_{PO} = \int_{hazard} P(PO | hazard) d(hazard)$$

- Must consider probability distributions of seismic hazard, of demand and of capacity due to:
 - Lack of knowledge (epistemic uncertainty)
 - Record-to-record randomness (aleatory uncertainty)

Seismic Hazard

- ◆ Use a ground motion intensity measures (PGA, $S_a(T1)$, etc.)
- ◆ Probability of exceeding a value of ground motion intensity (hazard curve):

$$P_H = H(s_a^{P_H}) = k_0 (s_a^{P_H})^{-k}$$

- ◆ An earthquake does not know if the structure is conventional or nuclear: seismic hazard is the same

Probability of Failure

◆ A comparison demand and capacity:

$$P_F = P(C \leq D) = \int_{s_a} P(F | s_a) |dH(s_a)|$$

◆ Key assumptions about hazard, demand and capacity probability distributions:

- Log-normal
- Dispersion β about the median

DOE-1020 and ASCE 43-05: Acceptance Criteria

- ◆ Probability of failure is smaller than probability of hazard
- ◆ Risk reduction ratio at the structure level

$$R_R = \frac{P_H}{P_F}$$

Performance Category	Risk Reduction Ratio
PC-1 (conventional)	$R_R=1.0$
PC-2 (internal exposure risk)	$R_R=1.0$
PC-3 (labs, fuel cycle facilities)	$R_R=10.0$
PC-4 (experimental reactors)	$R_R=20.0$

Conventional Design: Acceptance Criteria

◆ Probability of failure is, implicitly, assumed equal to the probability of hazard

$$P_F = P_H$$

◆ Design equation: ϕ

- Capacity reduction γ
- Demand amplification

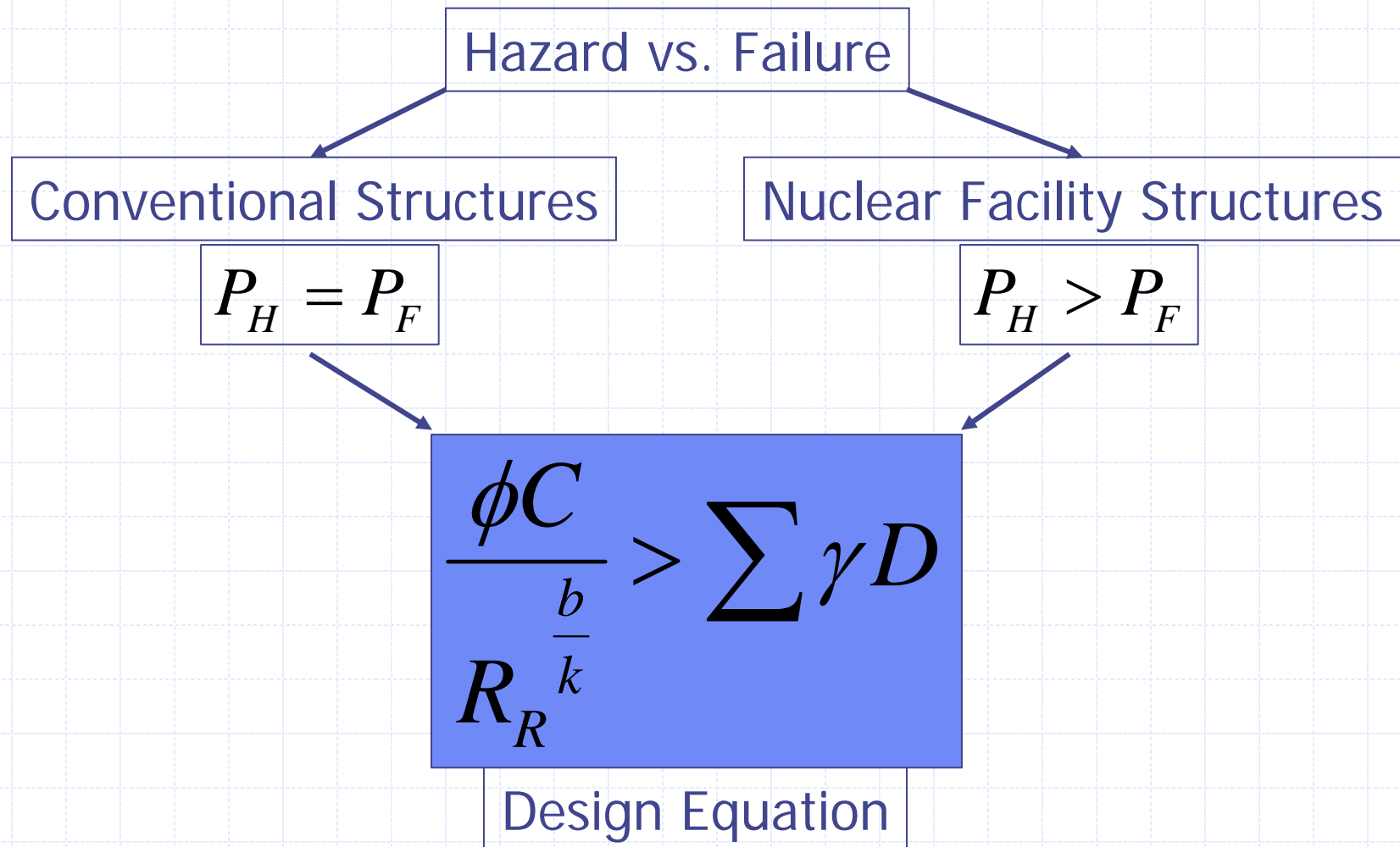
at the structural
element level

$$\phi C > \sum \gamma D$$

Two Formats: Unified

Probability of Hazard	$P_H = k_1 (S_a^{P_H})^k$	
IM Based Probability of Failure	$P_F = k_1 (\eta_{S_a, C})^{-k} e^{\frac{1}{2}(k \beta_C)^2}$	
PSDM Based Probability of Failure	$P_F = H_{S_a} (S_a^{\eta_C}) e^{\frac{1}{2} \frac{k^2}{b^2} \beta_{D S_a}^2} e^{\frac{1}{2} \frac{k^2}{b^2} \beta_C^2}$	
	Hazard Format	DFCD Format
Design/Assessment Equation	$R_R = \frac{P_H}{P_F}$	$\varphi \cdot \mathbf{C} \geq \gamma \cdot \mathbf{D}$
IM Based Assessment	$R_R = \left(\frac{P_o S_a}{\eta_{S_a, C}} \right)^{-k} e^{-\frac{1}{2}(k \beta_{S_a, C})^2}$	$\underbrace{P_o S_a}_{\text{Demand (D)}} = R_R^{-1/k} \underbrace{\eta_{S_a, C} e^{-\frac{1}{2} k \beta_{S_a, C}^2}}_{\text{Capacity } (\varphi C)}$
PSDM Based Assessment	$R_R = \left(\frac{P_o S_a}{S_a^{\eta_C}} \right)^{-k} e^{-\frac{1}{2} \frac{k^2}{b^2} (\beta_{D S_a}^2 + \beta_C^2)}$	$\underbrace{\eta_{D P_o S_a} e^{\frac{1}{2} \frac{k}{b} (\beta_{D S_a})^2}}_{\text{Demand } (\gamma D)} = R_R^{-b/k} \underbrace{\eta_C e^{-\frac{1}{2} \frac{k}{b} (\beta_C)^2}}_{\text{Capacity } (\varphi C)}$

Risk-Informed Design and Evaluation Framework

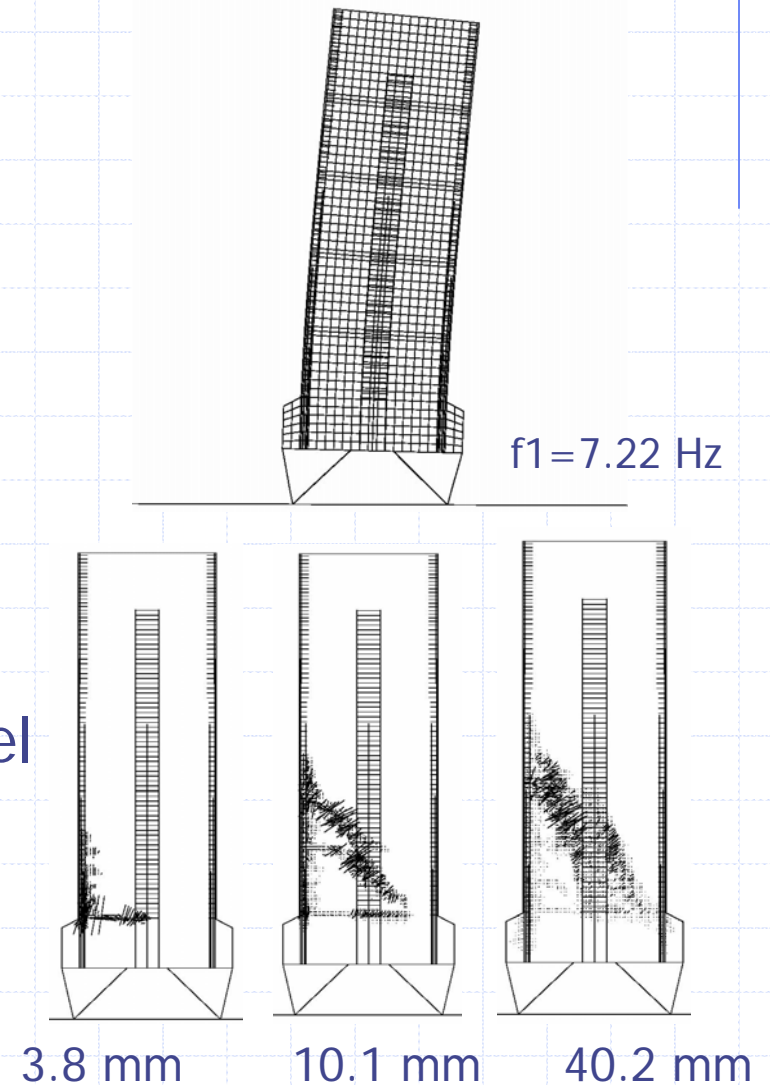


Simulation Needs

- ◆ Reduce epistemic uncertainty by improving our knowledge of how structures respond to earthquakes
- ◆ Reduce aleatory uncertainty by:
 - Improving estimates of seismic hazard
 - Measuring randomness in demand and capacity
- ◆ Formulate risk-informed evaluation framework:
 - Determine acceptable levels of risk reduction for performance levels relevant to nuclear facility structures

A CAMUS Shear Wall Example

- ◆ TNO DIANA 8.1
- ◆ 2-D shell-element model
- ◆ Included:
 - Shaking table model
 - Restraint provided by floor slabs
 - Embedded reinforcement
 - Interface elements to model construction joints

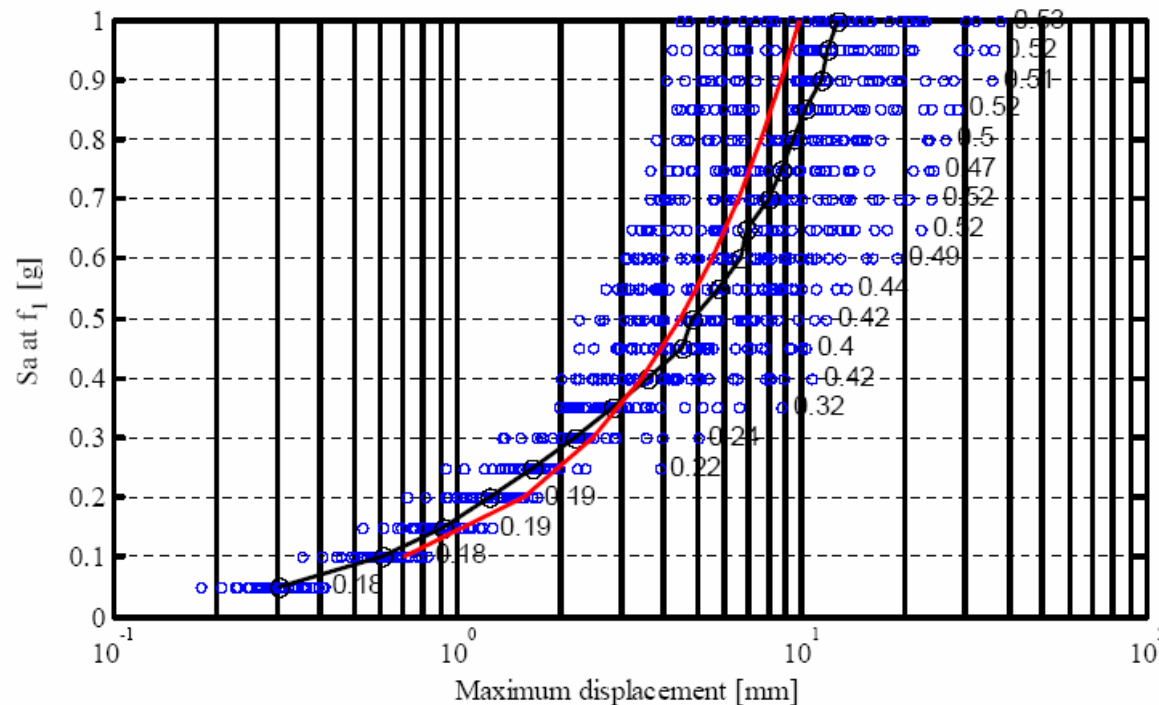


FEMA-356: Deterministic Acceptance Criteria

θ [rad]	demand	IO	LS	CP
Run 1	0.0014	0.002	0.004	0.008
Run 2	0.0005	0.002	0.004	0.008
Run 3	0.0021	0.002	0.004	0.008
Run 4	0.0017	0.002	0.004	0.008
Run 5	0.0045	0.002	0.004	0.008

Run 5: IO and LS limit states not satisfied
(IO – immediate occupancy)
(LS – life safety)

Probabilistic Demand Analysis



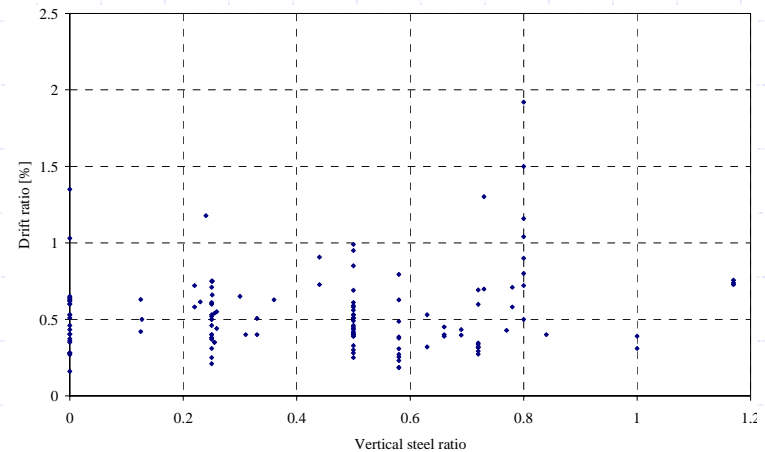
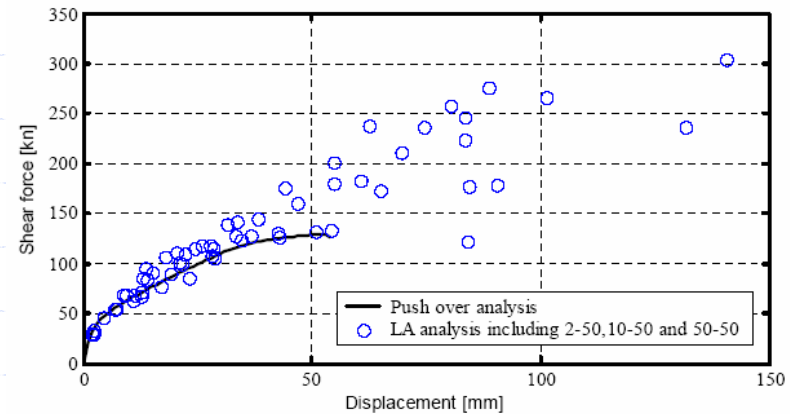
- ◆ A suite of representative ground motions
- ◆ Incremental Dynamic Analysis method
- ◆ Obtain median demand and dispersion

Probabilistic Capacity Analysis

◆ Done using:

- Numerical models of the wall
- Analysis of experimental data from tests on similar walls

◆ Provides a good estimate of the median capacity and dispersion



Risk-Informed Approach

- ◆ Computing risk reduction afforded by the CAMUS wall is now possible
 - This value is about 5 for the considered (Western Europe) seismic risk environment and the structural collapse limit state
- ◆ Finding: the wall is not adequate for PC-3 and PC-4
 - We know by how much
 - We know where reducing uncertainty will be most effective

Conclusion

- ◆ Modern structural design is based on a probabilistic consideration of failure:
 - Nuclear facility design provisions are formulated on the structure level
 - Conventional structures design provisions are formulated on the element level
- ◆ It is possible to formulate a unified, risk-informed design approach
- ◆ The unified approach enables using advances in earthquake engineering of conventional structures for seismic design and evaluation of nuclear facility structures



Thank you!

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